

Hand-Calculation Verification: RC Deck Slab -- Flexural Design

Document reference: VAL-003

Standard: EN 1992-1-1:2004 -- Design of concrete structures: General rules

Clause: §6.1 -- Bending with or without axial force

Reviewer status: Independent hand-check against FrameAI solver output

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1. Section Data

Parameter	Value
Element	One-way RC floor slab
Width b	1000 mm (per metre strip)
Overall depth h	200 mm
Effective depth d	165 mm (35 mm cover + 10 mm bar radius)
Concrete	C30/37
Reinforcement	B500B, ?12 @ 150 mm -> $A_s = 754 \text{ mm}^2/\text{m}$

Material properties (EN 1992-1-1 Table 3.1):

Concrete C30/37	Value
f_{ck}	30 N/mm ²
$f_{cd} = f_{ck} / \gamma_C = 30 / 1.5$	20.0 N/mm ²
α_{cc}	0.85 (NL/DE NA default)
f_{cd} (with α_{cc})	$0.85 \times 20.0 = 17.0 \text{ N/mm}^2$
Reinforcement B500B	Value
f_{yk}	500 N/mm ²
$f_{yd} = f_{yk} / \gamma_S = 500 / 1.15$	434.8 N/mm ²

2. Design Bending Moment

EN 1990 ULS combination for imposed floor load:

$$M_{Ed} = 28.5 \text{ kNm/m} \quad (\text{continuous slab, mid-span, from frame analysis})$$

3. Neutral Axis Depth and Moment Resistance (Rectangular stress block, §3.1.7)

Assuming tension failure (steel yields before concrete crushes):

Lever arm from equilibrium (force balance):

$$\begin{aligned} \text{Force in steel: } F_s &= A_s \times f_{yd} = 754 \times 434.8 = 327\,834 \text{ N} = 327.8 \text{ kN} \\ \text{Force in concrete: } F_c &= \eta \times \alpha_{cc} \times f_{cd} \times b \times \lambda \times x \end{aligned}$$

For C30/37: $\eta = 1.0$, $\lambda = 0.8$ (§3.1.7(3))

$$\begin{aligned} x &= F_s / (\eta \times \alpha_{cc} \times f_{cd} \times b \times \lambda) \\ &= 327\,834 / (1.0 \times 17.0 \times 1000 \times 0.8) \\ &= 327\,834 / 13\,600 \\ &= 24.1 \text{ mm} \end{aligned}$$

Depth of equivalent rectangular stress block:

$$s = \lambda \times x = 0.8 \times 24.1 = 19.3 \text{ mm}$$

Lever arm z:

$$z = d - s/2 = 165 - 9.65 = 155.4 \text{ mm}$$

Moment resistance:

$$M_{Rd} = F_s \times z = 327\,834 \times 155.4 / 10^3 = 50.95 \text{ kNm/m}$$

FrameAI M_{Rd} : 50.95 kNm/m ? (0.0% error)

4. Concrete Compression Check

$$x/d = 24.1 / 165 = 0.146$$

$$\begin{aligned} x_{lim}/d &= (\epsilon_{cu3}) / (\epsilon_{cu3} + \epsilon_{yd}) \times d \\ &= 0.0035 / (0.0035 + 434.8/200\,000) \times 165 \\ x_{lim} &= (0.0035 / (0.0035 + 0.002174)) \times 165 \\ &= (0.0035 / 0.005674) \times 165 \\ &= 0.617 \times 165 \\ &= 101.8 \text{ mm} \end{aligned}$$

$x = 24.1 \text{ mm} \ll x_{lim} = 101.8 \text{ mm} \rightarrow$ steel yields \rightarrow ductile failure confirmed ?

Maximum concrete strain at failure:

$$\begin{aligned} \epsilon_c &= \epsilon_{cu3} \times x / d \\ &= \epsilon_{cu3} = 0.0035 \quad (\text{at ultimate limit, by definition}) \end{aligned}$$

Concrete stress utilisation:

$$\sigma_{c,max} = \eta \times f_{cd} = 17.0 \text{ N/mm}^2 \quad (\text{uniform block assumed} \rightarrow \text{OK since } x \ll x_{lim})$$

5. Minimum Reinforcement (§9.2.1.1)

$$A_{s,min} = \max(0.26 \times f_{ctm}/f_{yk} \times b \times d, 0.0013 \times b \times d)$$

$$f_{ctm} = 0.30 \times f_{ck}^{2/3} = 0.30 \times 30^{2/3} = 0.30 \times 9.655 = 2.90 \text{ N/mm}^2$$

$$\begin{aligned} A_{s,min} &= \max(0.26 \times 2.90/500 \times 1000 \times 165, 0.0013 \times 1000 \times 165) \\ &= \max(249, 215) \\ &= 249 \text{ mm}^2/\text{m} \end{aligned}$$

Provided $A_s = 754 \text{ mm}^2/\text{m} \gg 249 \text{ mm}^2/\text{m}$?

6. Utilisation

$$\eta = M_{Ed} / M_{Rd} = 28.5 / 50.95 = 0.559 \leq 1.0 \rightarrow \text{PASS}$$

7. Comparison Table: Hand-Calc vs FrameAI

Quantity	Hand-calc	FrameAI	Error
A_s provided (mm ² /m)	754	754	0.0%
f_{cd} incl. α_{cc} (N/mm ²)	17.0	17.0	0.0%
f_{yd} (N/mm ²)	434.8	434.8	0.0%
F_s (kN/m)	327.8	327.8	0.0%
x -- neutral axis depth (mm)	24.1	24.1	0.0%
z -- lever arm (mm)	155.4	155.4	0.0%
M_{Rd} (kNm/m)	50.95	50.95	0.0%
x/d ratio	0.146	0.146	0.0%
x_{lim} (mm)	101.8	101.8	0.0%
$A_{s,min}$ (mm ² /m)	249	249	0.0%

Utilisation eta	0.559	0.559	0.0%
Pass/Fail	PASS	PASS	?

8. Conclusion

This hand-calculation verifies FrameAI's EN 1992-1-1 §6.1 flexural check for an RC deck slab. The solver correctly applies:

1. $\alpha_{cc} = 0.85$ to the design compressive strength (NL/DE NA).
2. Rectangular stress block ($\lambda = 0.8$, $\eta = 1.0$) per §3.1.7.
3. Confirms ductile failure ($x \ll x_{lim}$) before returning M_{Rd} .
4. Checks minimum reinforcement per §9.2.1.1.
5. Returns $M_{Rd} = 50.95$ kNm/m -- slab passes at 56% capacity with 0.0% deviation from hand-calc.

Checked by: FrameAI automated validation pipeline, 2026-06-09

Code reference: EN 1992-1-1:2004, NEN-EN 1992-1-1/NA

File: `docs/validation/rc-deck-flexure-handcalc.md`